

FIELD TESTING AND PERFORMANCE OF METAL CULVERTS IN OHIO

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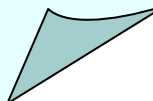


October 23-24 , 2007

RESEARCH OBJECTIVES & OUTLINE

- ODOT maintains thousands of culverts in Ohio
- In-house program *CMP-Excel* is used for evaluation

Objectives:

- Review ODOT's culvert database
(Parameters: geometry, loading, age, soil depth etc.)
 - Perform field testing of 40 culverts in Ohio
 - Validate and verify *CMP-Excel* using test data
 - Model and analyze culverts (e.g., CANDE, FLAC)
 - Compare the test data with analytical results
 - Propose new methods/recommendations
- 

DO CULVERTS FAIL? CONSEQUENCES



Vail, CO



Bakersfield, CA



Cuyahoga Co, Ohio

RESEARCH APPROACH

Experimental Research:

- **Develop a test matrix**
 - **Instrumentation: strains and displacements**
 - **Loading: standard loading truck**
 - **Static and dynamic tests**

Analytical Research:

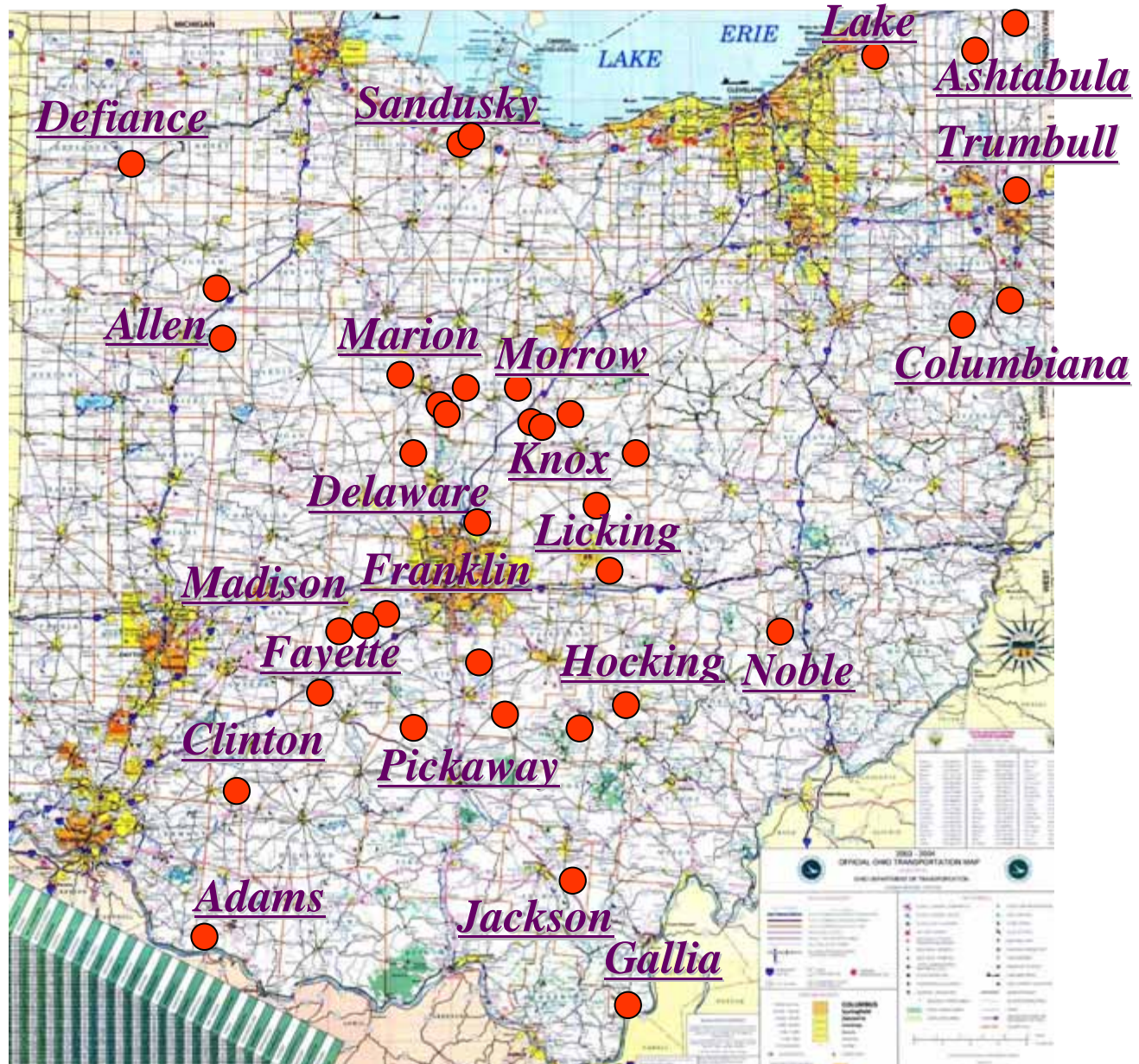
- **Model and analyze the test culverts**
 - **use simplified 2-D models (CANDE)**
 - **equivalent line loading (AASHTO)**
 - **use detailed 3-D models (FLAC-3D)**
- **Evaluate the in-house program, *CMP-Excel***

TEST CULVERTS

Number of culverts tested so far: 38

- **Backfill height: 0.9 to 24.5 ft**
- **Span length: 11 to 21 ft**
- **General appraisal number: 2 to 9**
- **ADT: 580 - 12420**
- **Year built: 1938 - 1997**
- **Different test locations in Ohio**

TEST SITE LOCATIONS



INSTRUMENTATION

Direct displacement measurements (inside culvert):

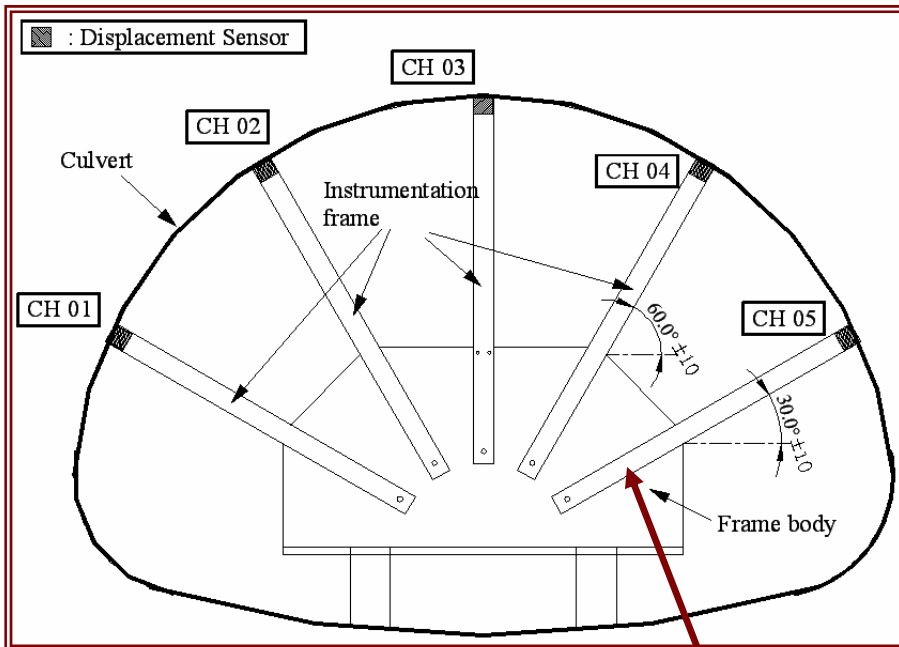
- Vertical and diagonal directions
- Displacement sensors attached on an adjustable frame
- 5 channels (CH 01 - CH 05)
- CH06 is used for slippage between culvert plates

Strain measurements:

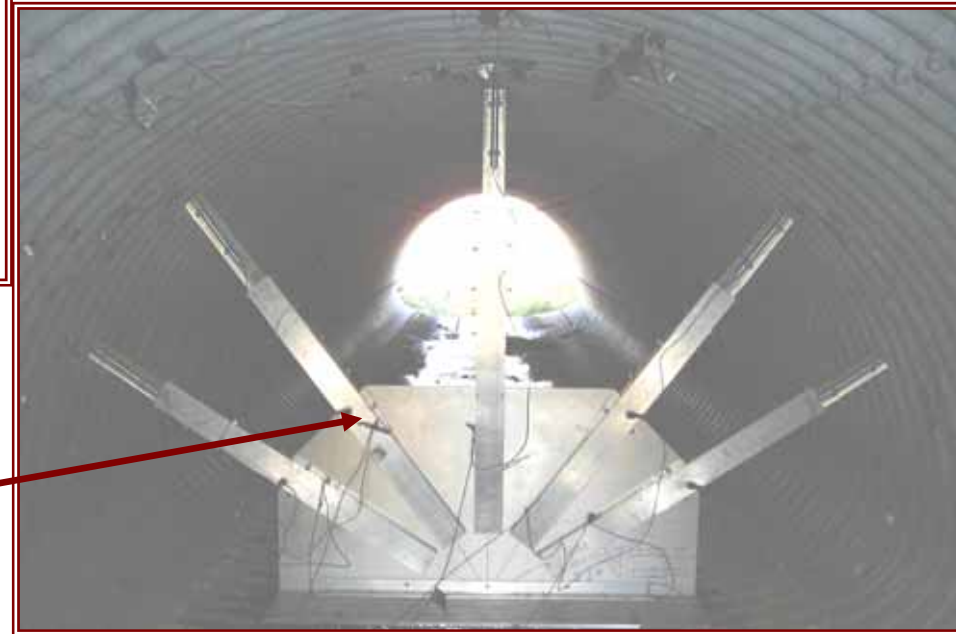
- Information about strains and stresses
- Strain gauges attached on the inside surface of culvert
- Longitudinal and transverse directions
- 14 channels (CH 07 - CH 20)

DISPLACEMENT MEASUREMENTS:

Locations of the 5 displacement sensors (CH 01 – CH 05)

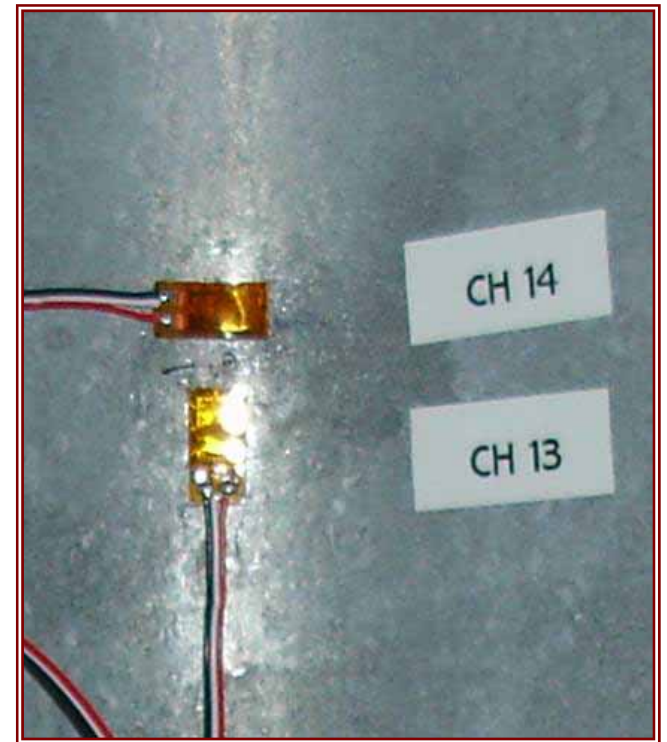
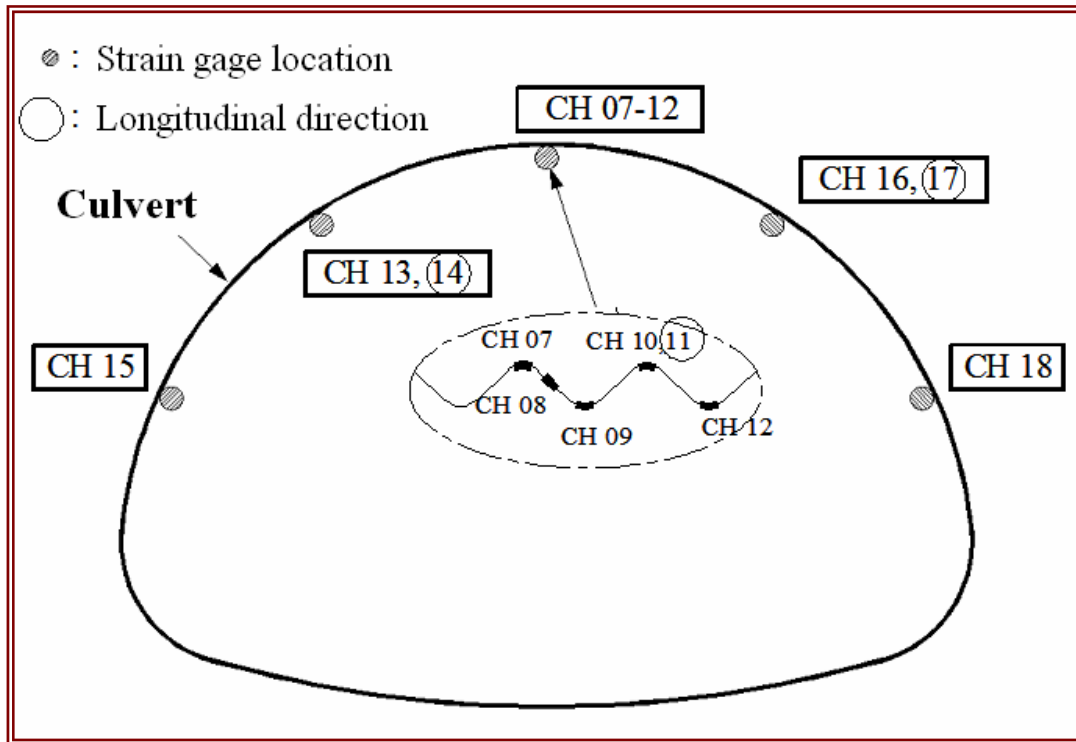


**Adjustable
instrumentation
frame**



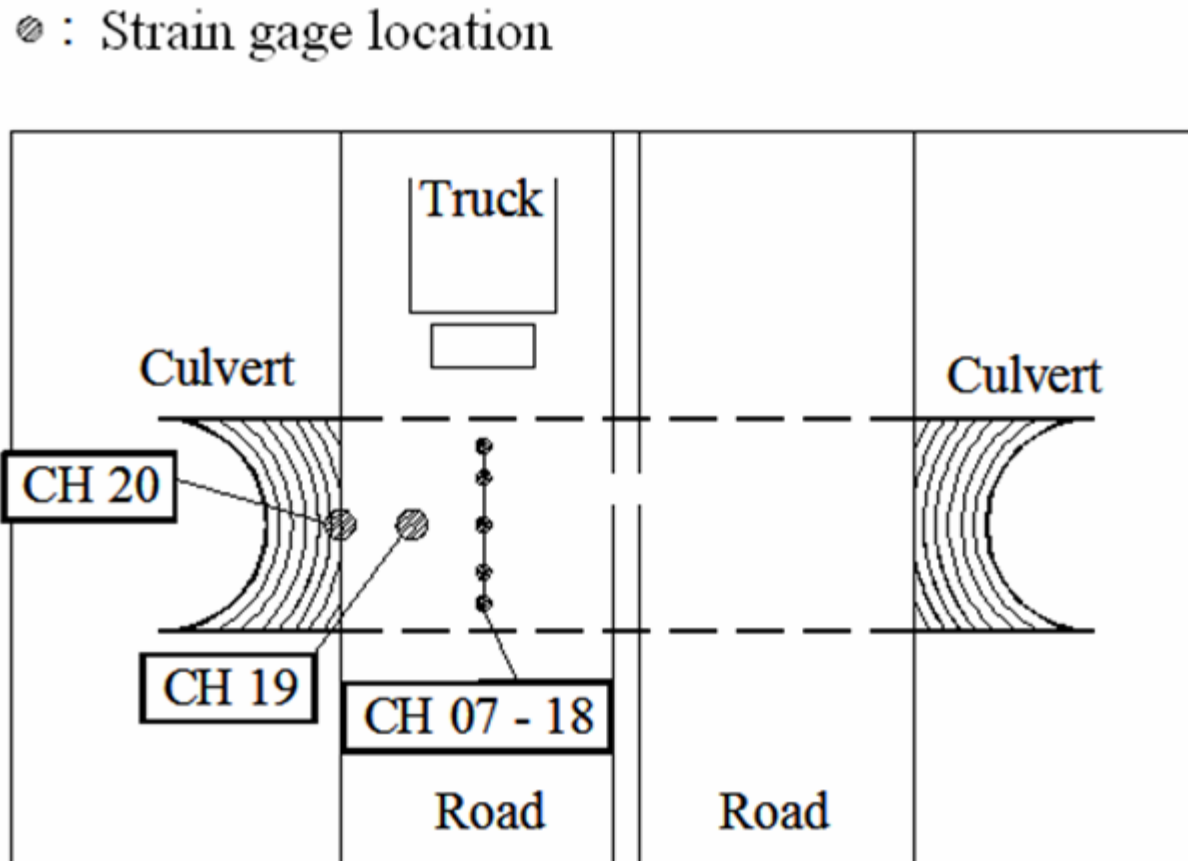
STRAIN MEASUREMENTS

Locations of the 12 strain gages (CH 07 – CH 18)



STRAIN MEASUREMENTS

Gauge locations along culvert length (plan view): CH 07–CH 20

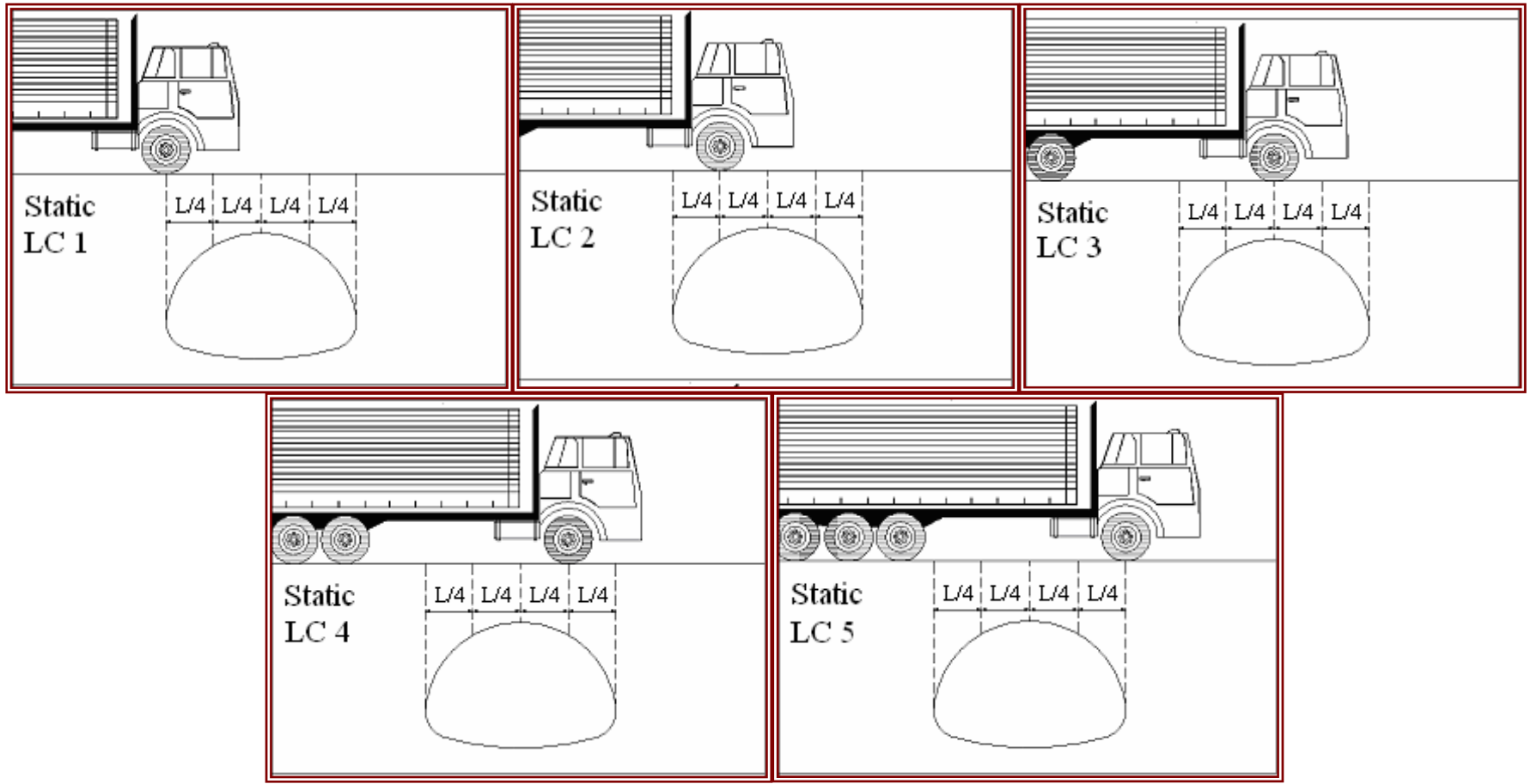


LOADING TRUCK

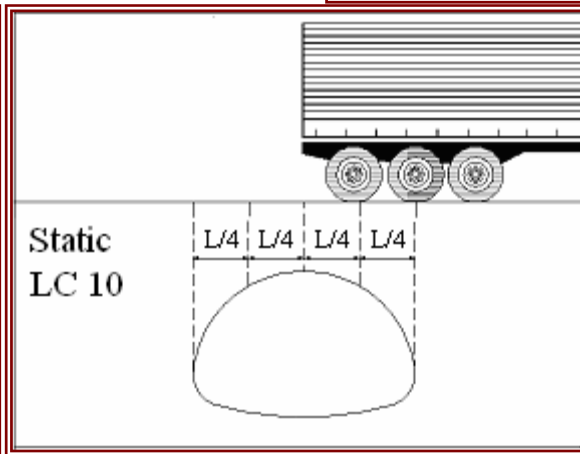
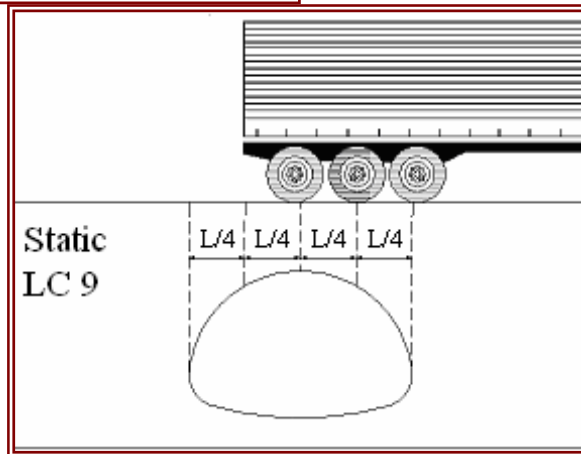
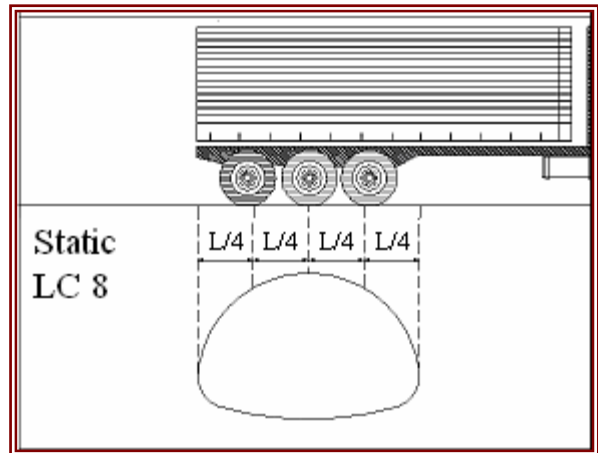
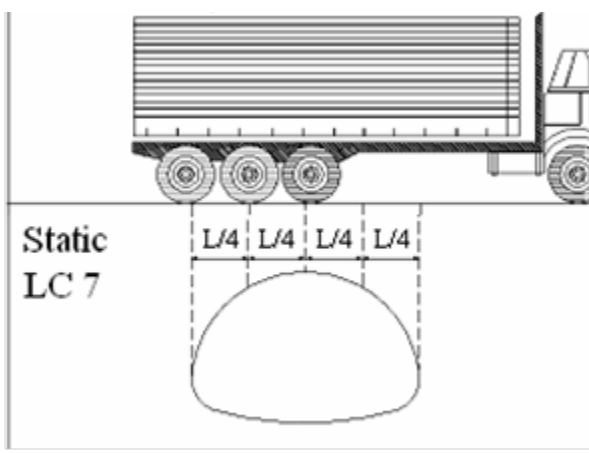
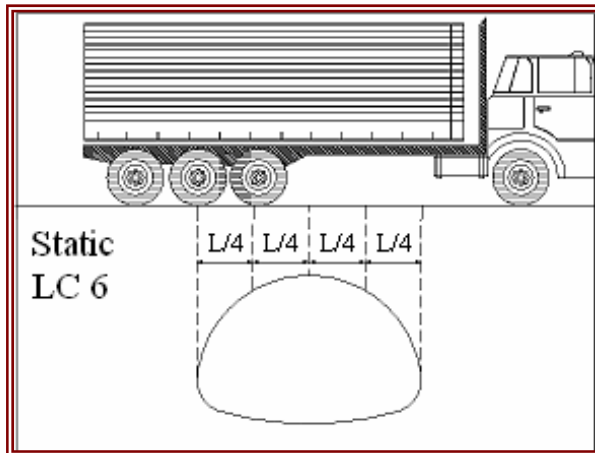


Total weight: 54-89 kips
Steering axle: 12-27 kips
Drive axles: 16-31 kips

STATIC LOAD POSITIONS



STATIC LOAD POSITIONS



STATIC TESTS

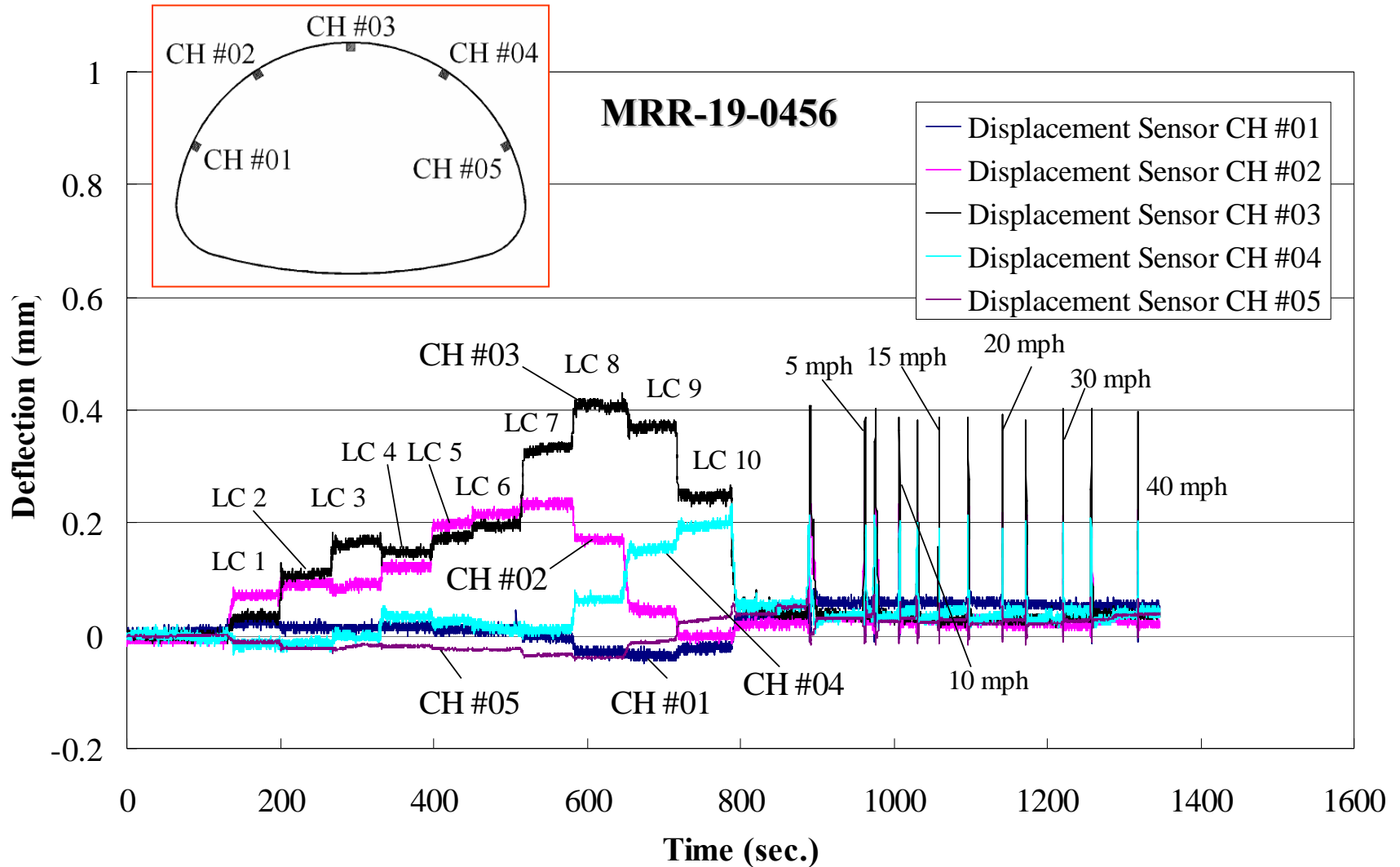
- **Maximum displacement: 2.25 mm (0.09 in.)**
(very shallow culvert - 1 ft deep)
- **Maximum strain: 108×10^{-6}**
(very low appraisal number, 3)

DYNAMIC LOADING

Truck driving over the culvert at speeds:

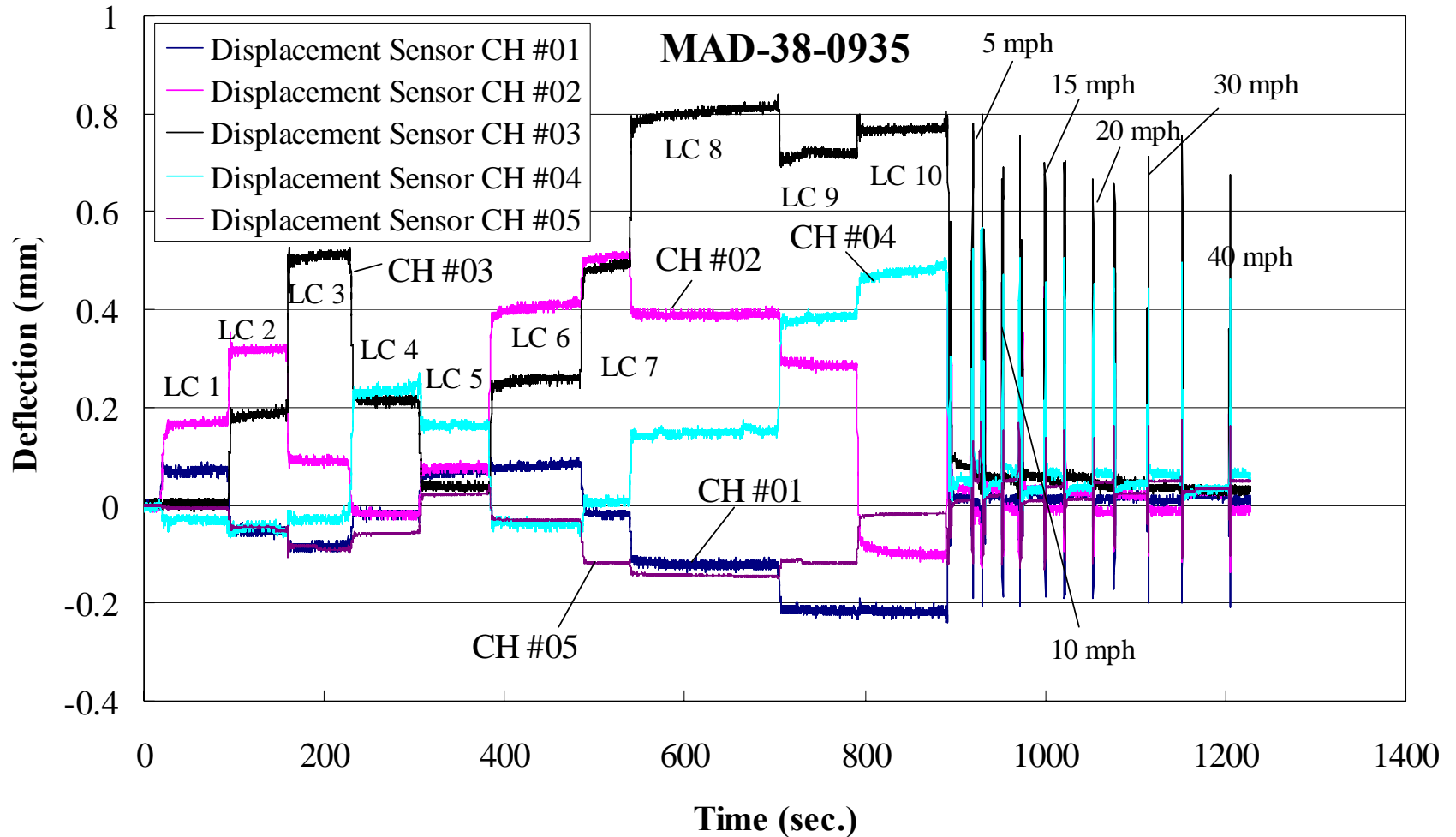
- 5 - 10 – 15 – 20 – 25 – 30 mph
- Legal speed limit

SAMPLE TEST RESULTS



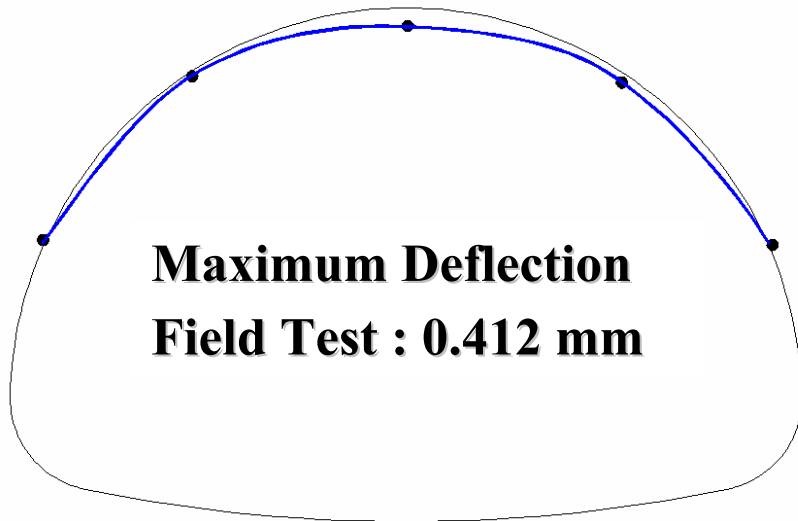
Route 19, SLM 0456, Morrow County

SAMPLE TEST RESULTS

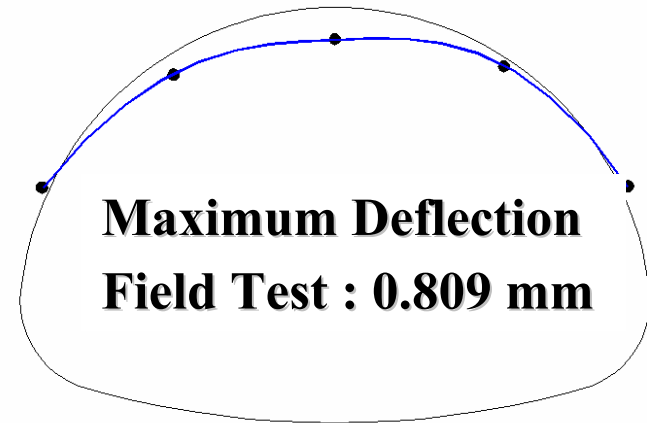


Route 38, SLM 0935, Madison County

DEFORMED SHAPES OF TWO CULVERTS

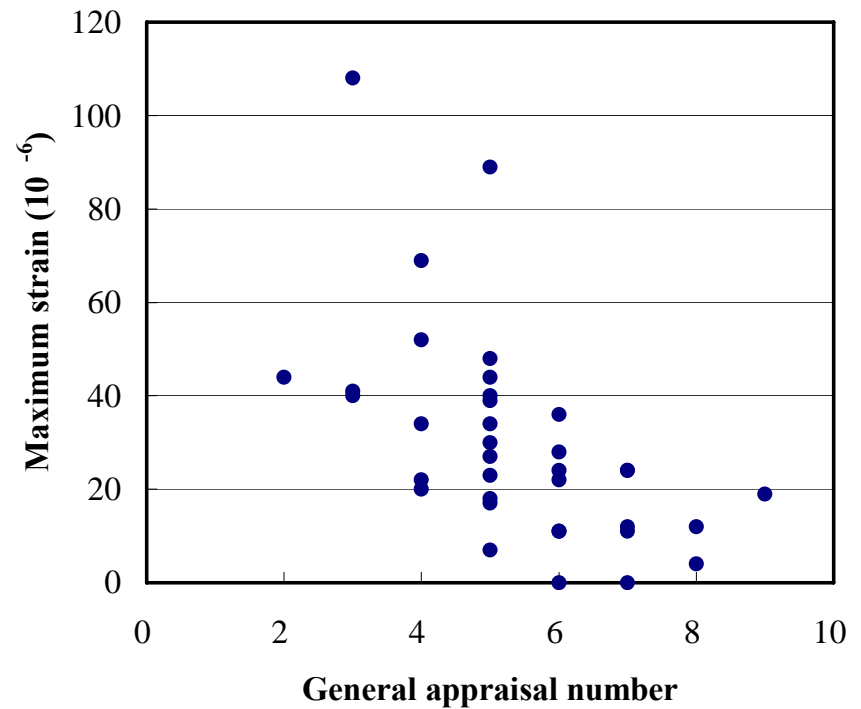
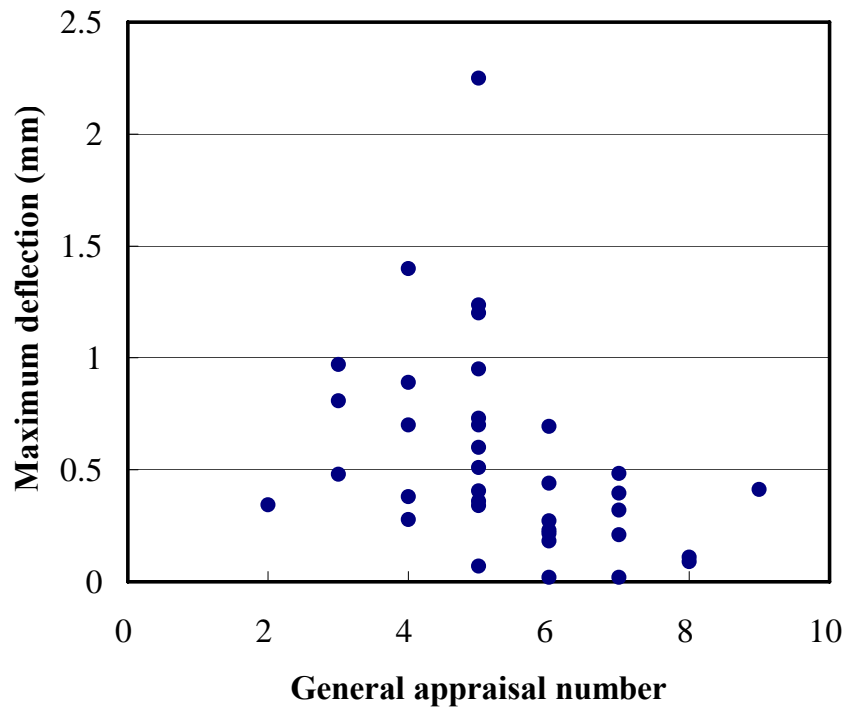


**Route 19, SLM 0456,
Morrow County**

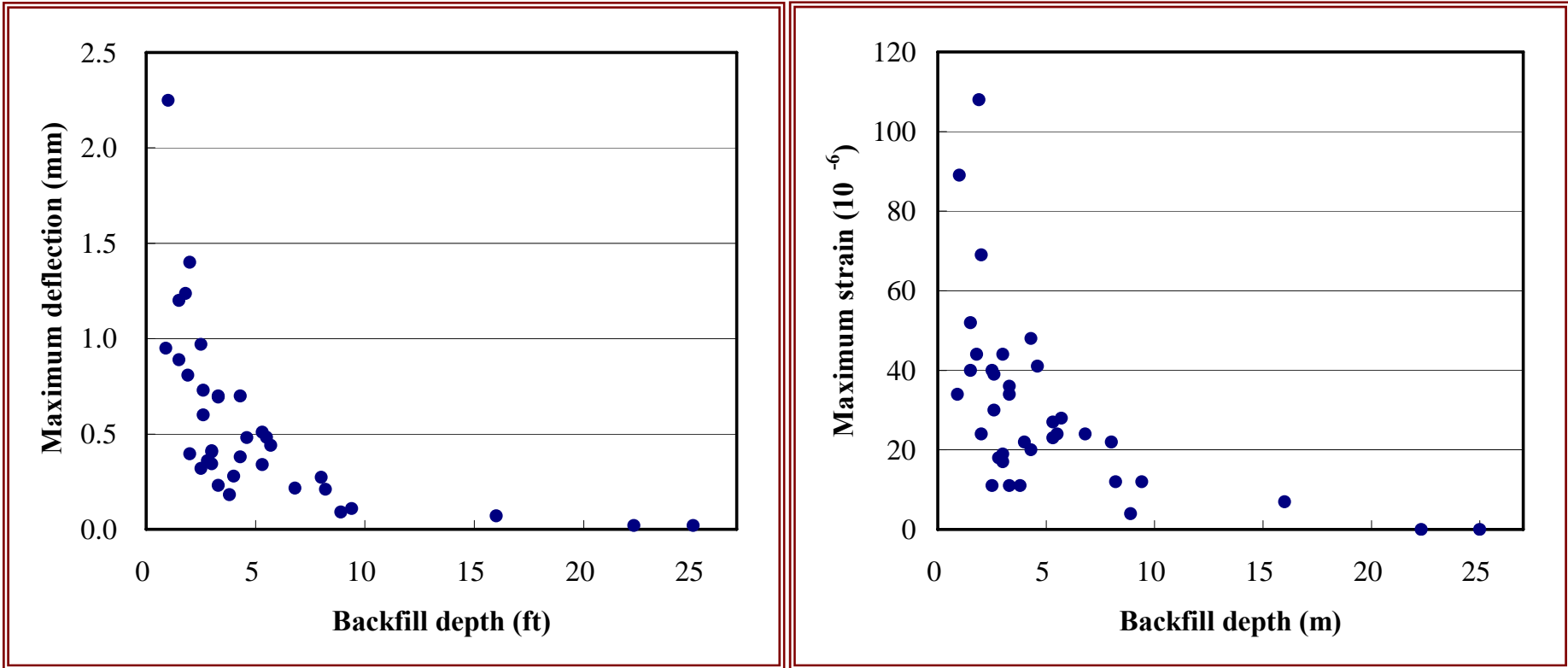


**Route 38, SLM 0935
Madison County**

Deformation vs. General Appraisal Number

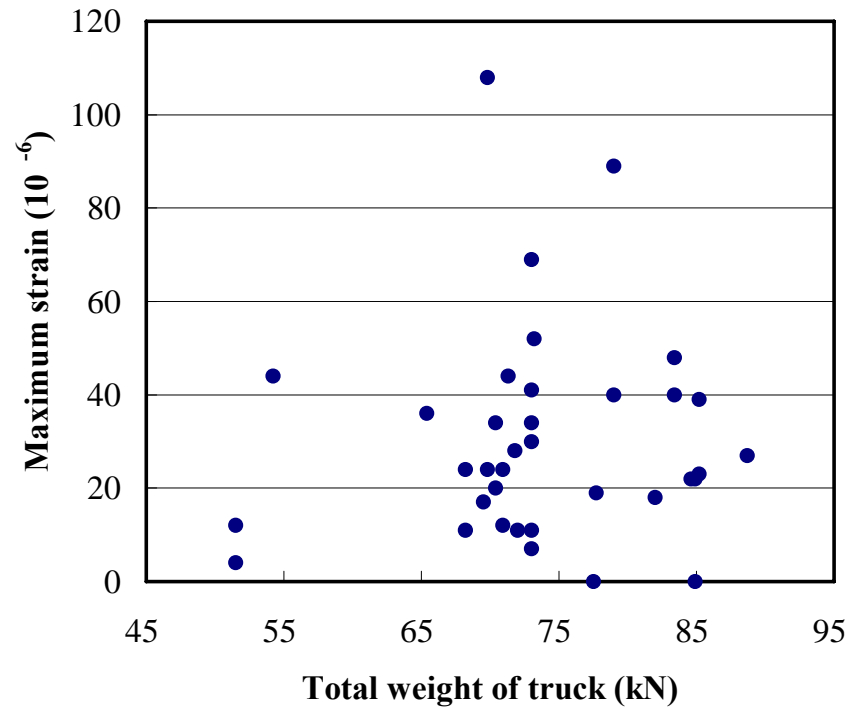
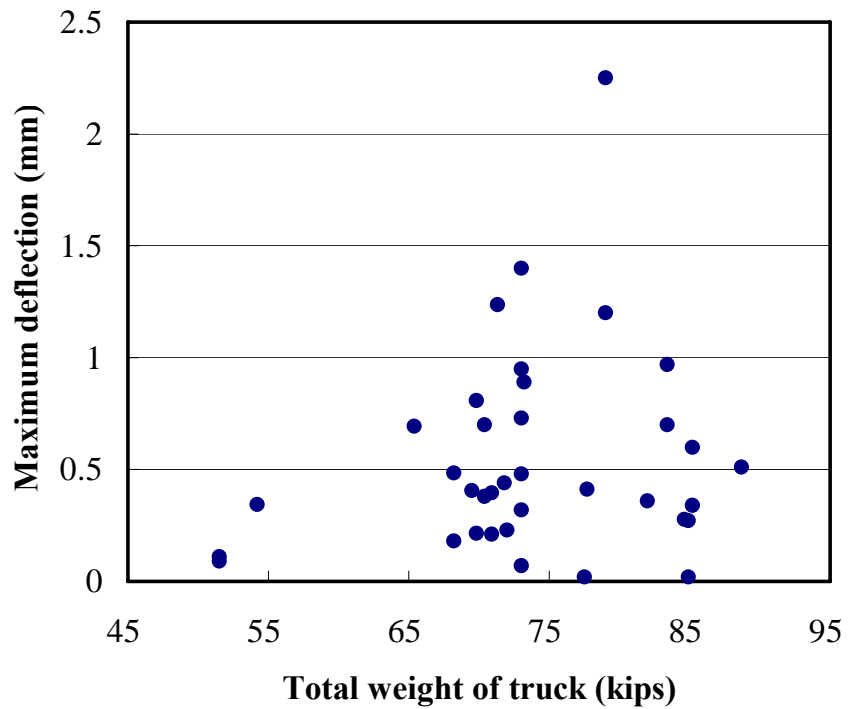


EFFECT OF BACKFILL DEPTH



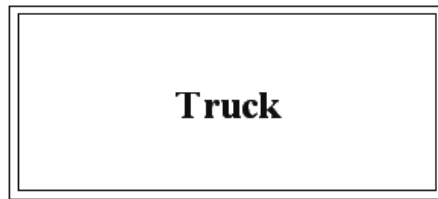
- Deformations seem to decrease with increasing backfill
- Very deep culverts do not feel the load

EFFECT OF TOTAL TRUCK WEIGHT



Equivalent Line Load (AASHTO)

Width of wheels (W) D_w Width of wheels (W)



Conversion of total axle load, Q to line load, q :

$$q = \frac{Q}{1.75H + D_w + 2W}$$

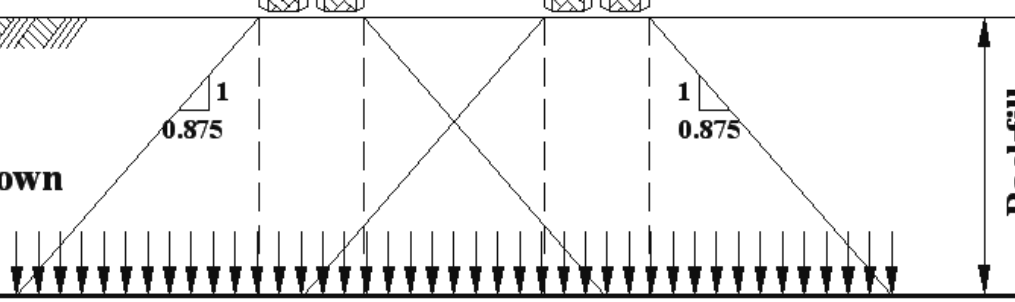
Backfill height (H)

1
0.875

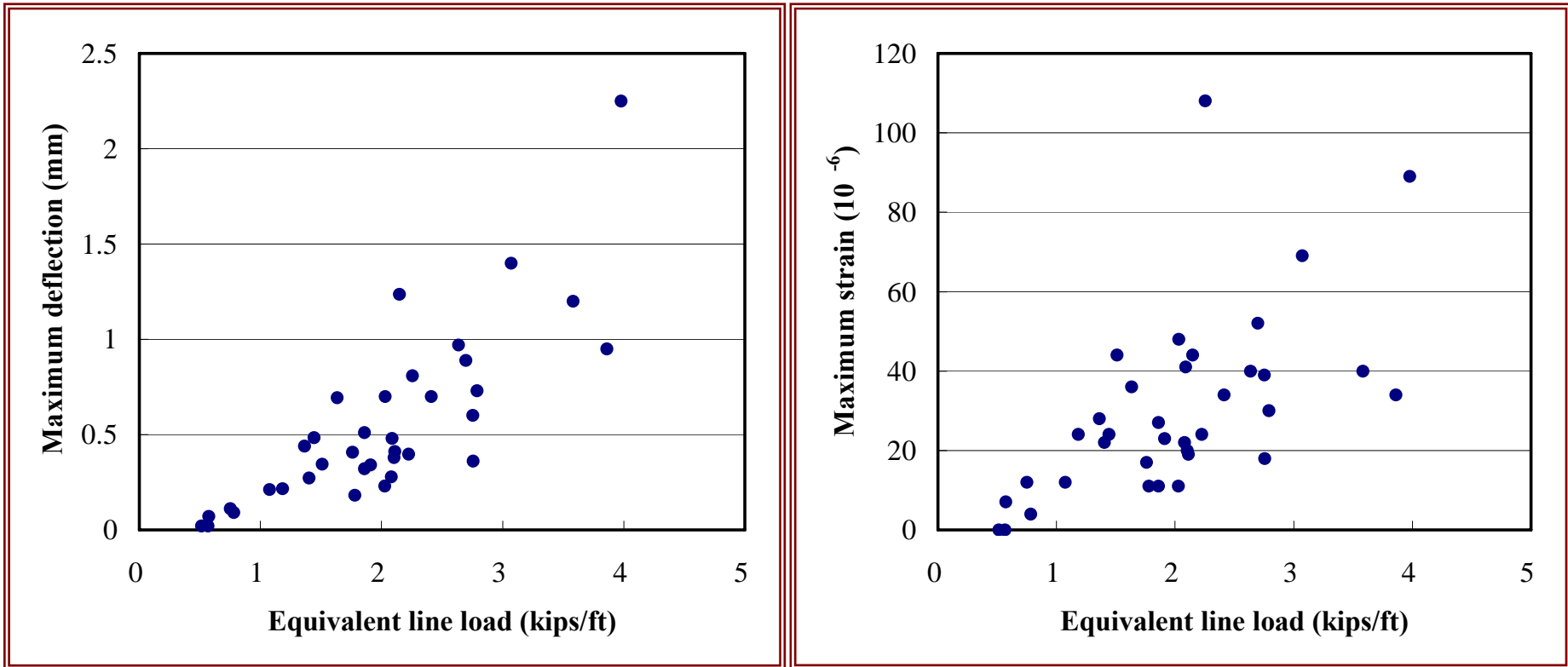
1
0.875

Culvert crown

$1.75H + 2W + D_w$

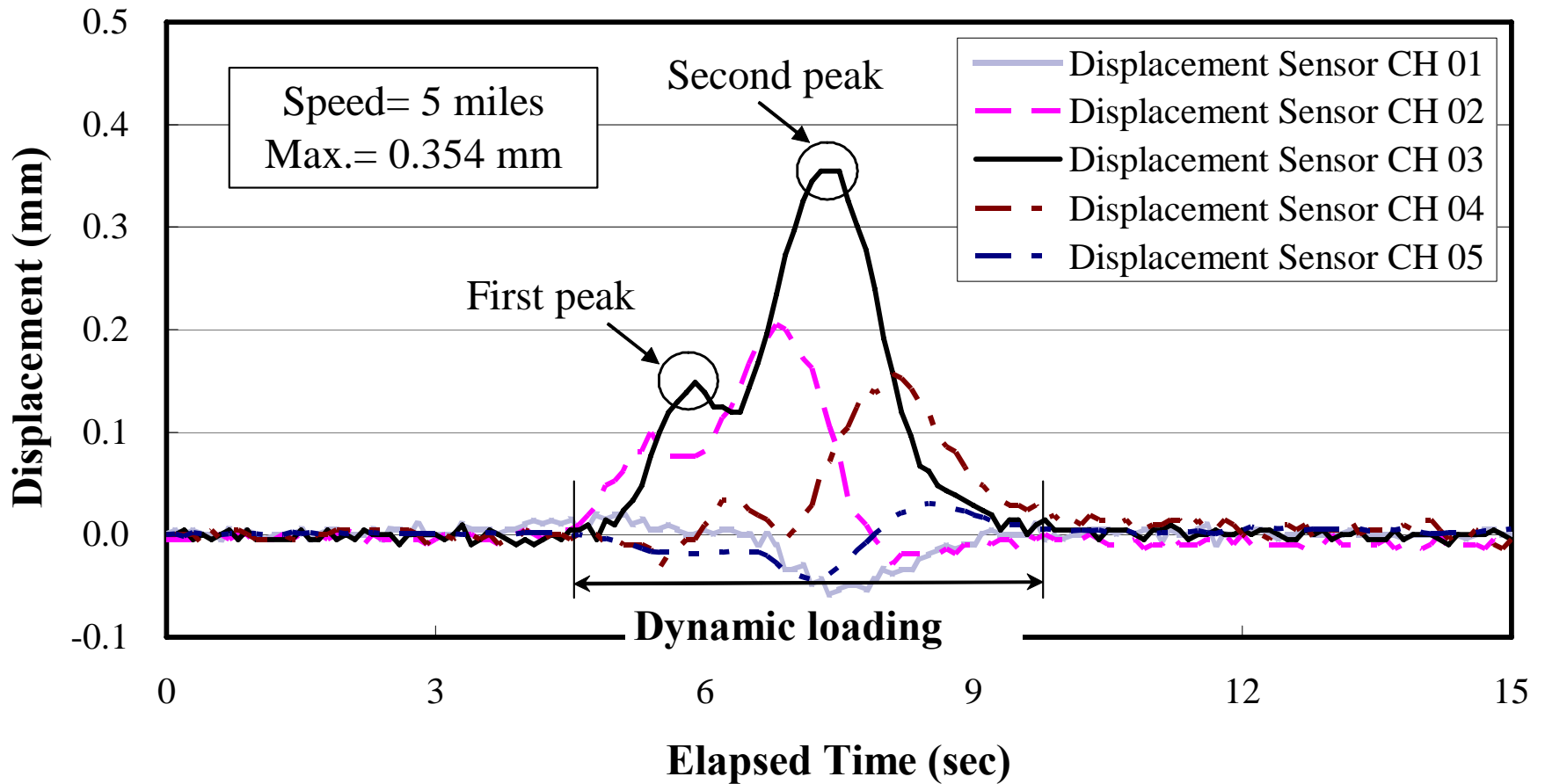


Effect of Equivalent Line Load (AASHTO)

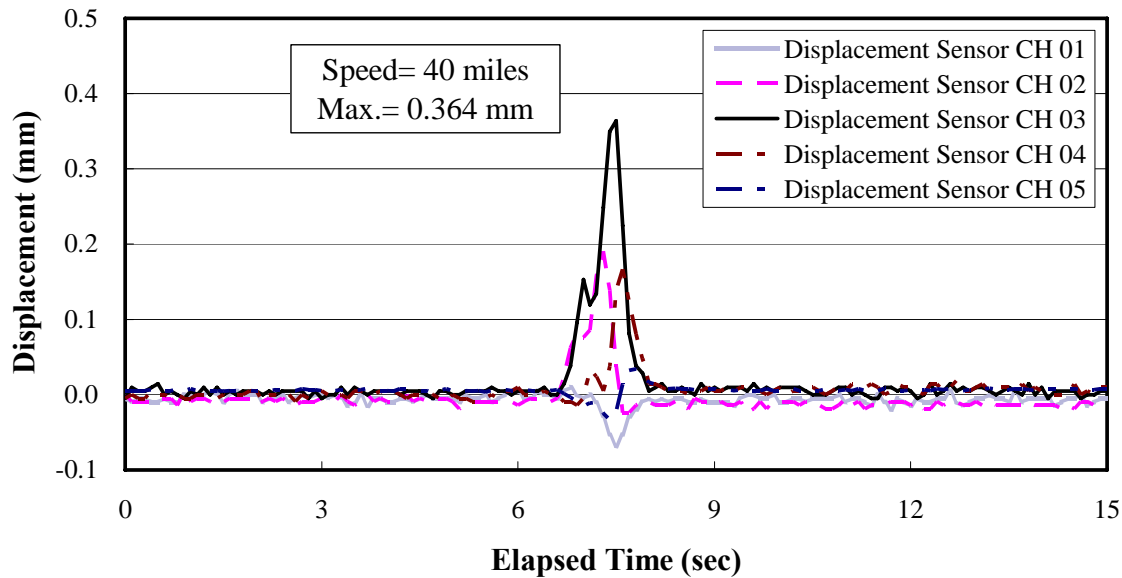
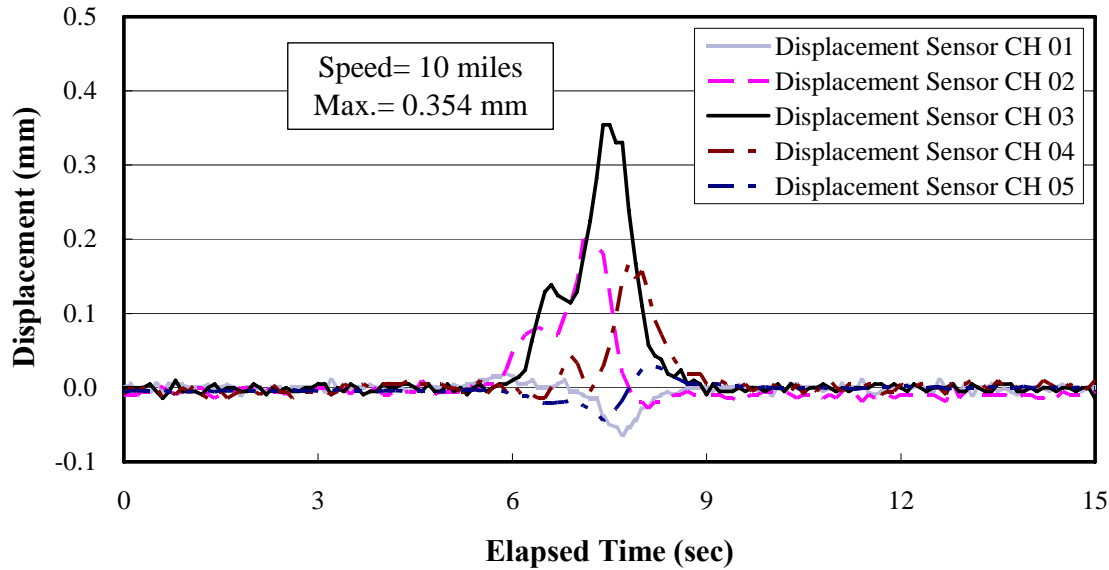


- Equivalent line load includes the effects of truck load and backfill height

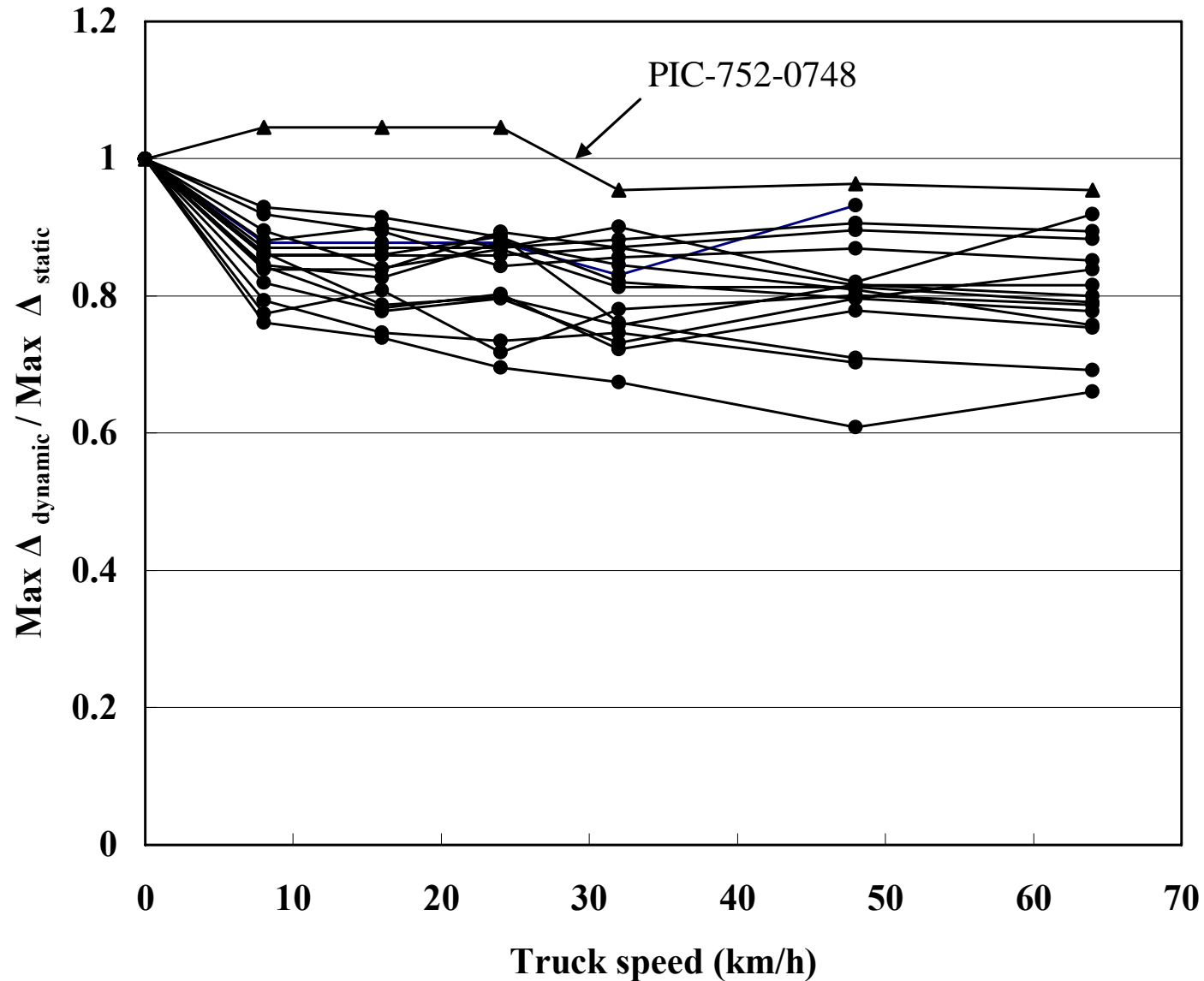
DYNAMIC LOAD TESTS



DYNAMIC TEST DATA



Maximum Deflection Ratio vs. Truck Speed



ANALYTICAL RESEARCH

2-D Modeling of culverts using CANDE program

→ **Plane-strain analysis**

→ **Comparison of analysis results with field test data**

3-D Modeling of culverts using FLAC program

→ **FLAC: Fast Lagrangian Analysis of Continua**

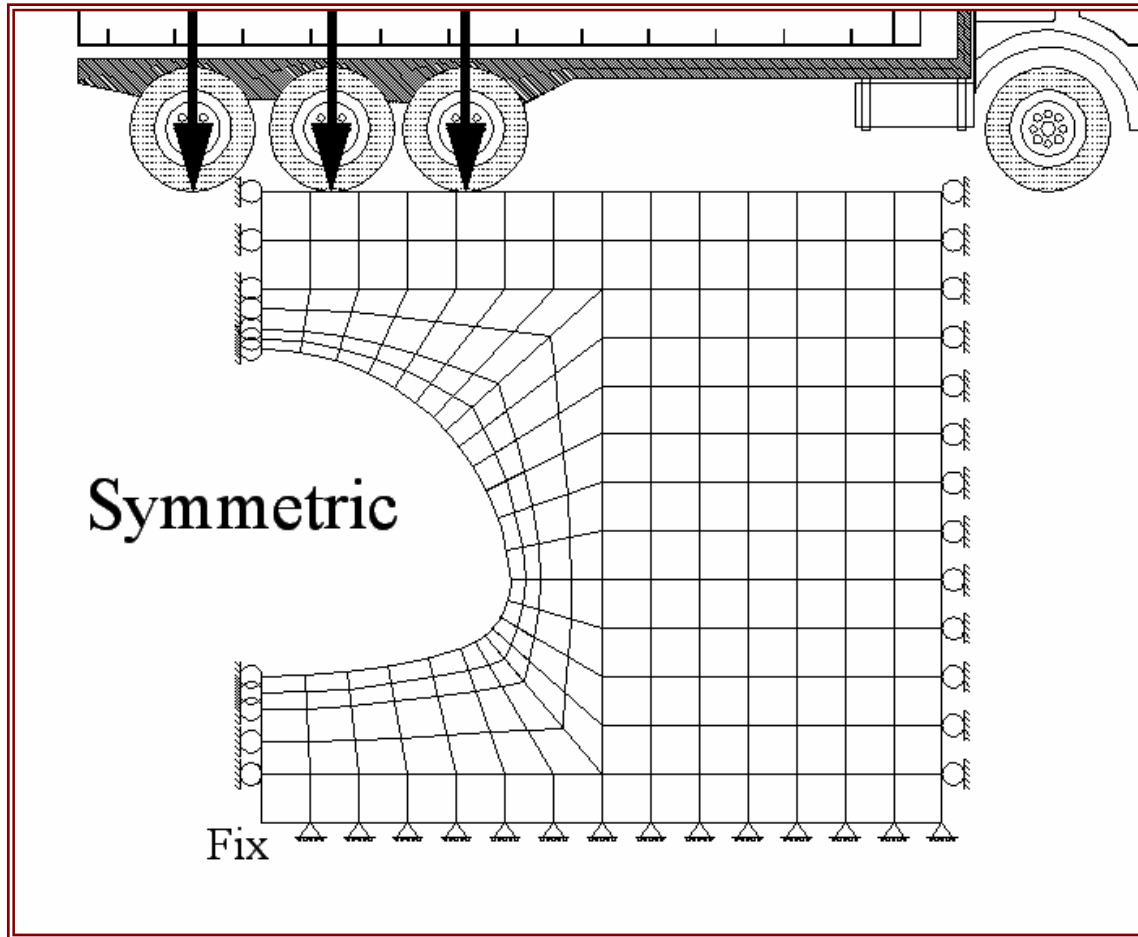
→ **3-D analysis including the soil-structure interaction**

Evaluation of ODOT's in-house spreadsheet, CMP-Excel

→ **Load rating evaluation program based on AASHTO & NSCSPA**

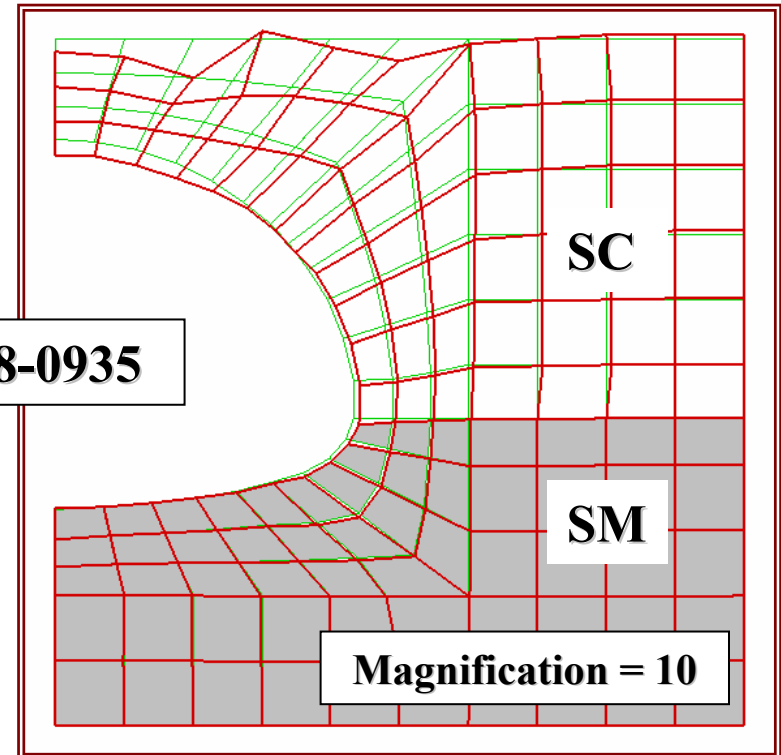
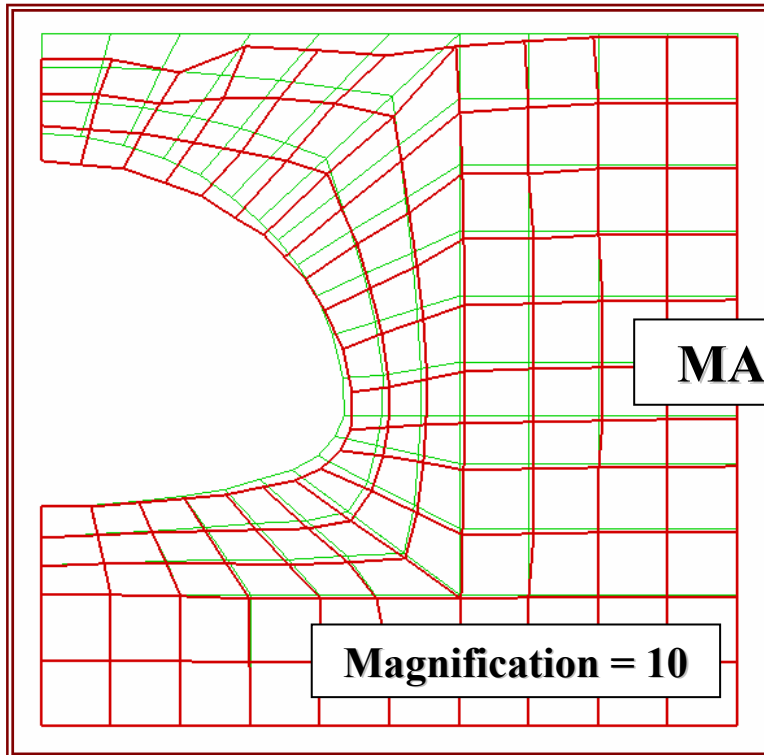


2-D CANDE MODEL



- Half model because of symmetry
- Roller boundary conditions on both sides
- Hinge boundary conditions at the bottom

CANDE ANALYSIS RESULTS

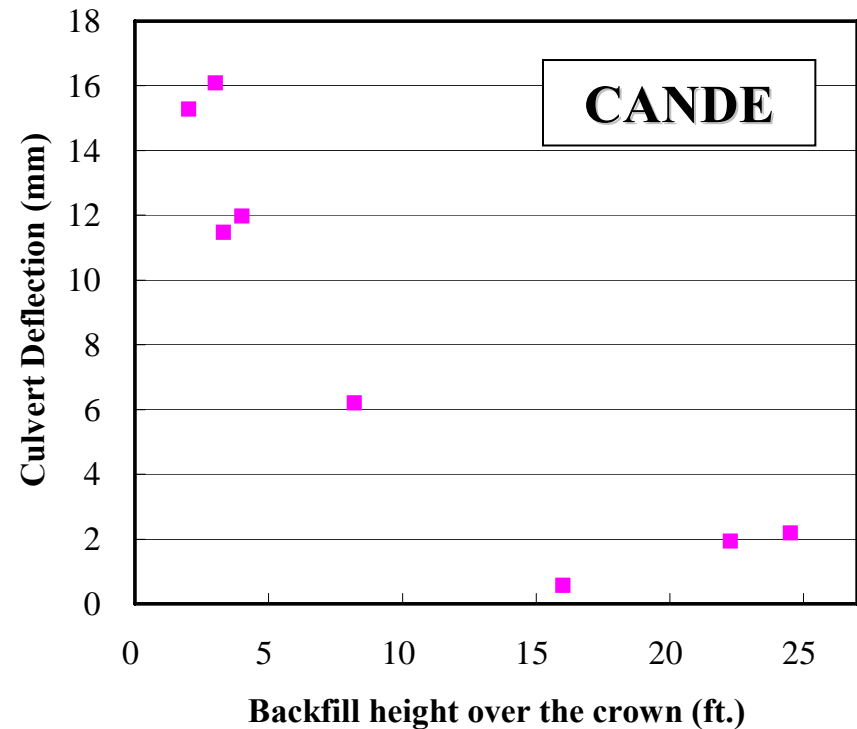
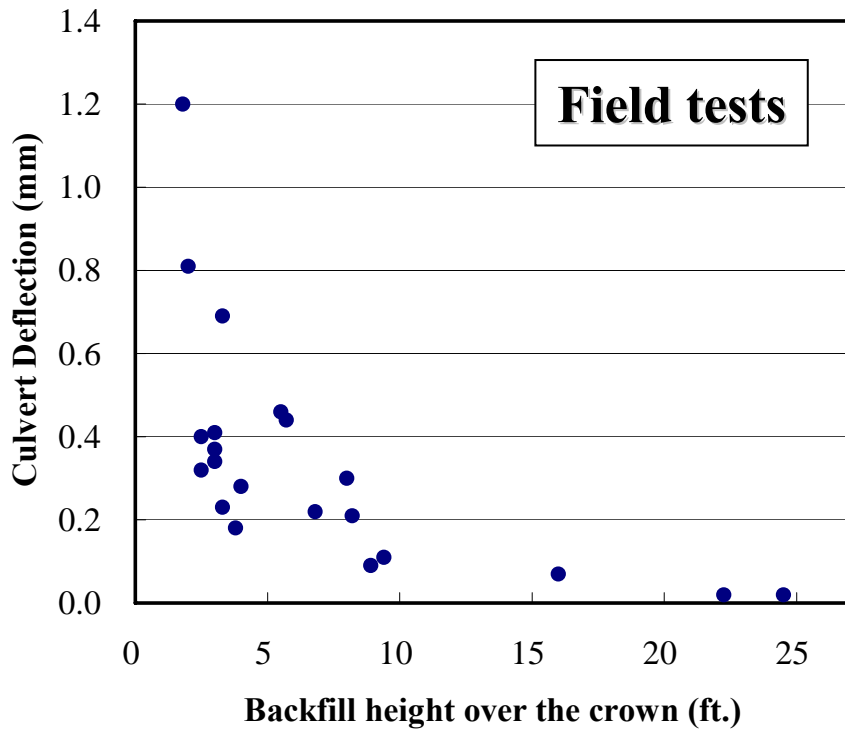


- Silty clayey sand (SC)
- Relative compaction: 100%
- Maximum displ.: 15.28 mm

- SC + Silty sand (SM)
- Relative compaction: 100%
- Maximum displ.: 9.59 mm

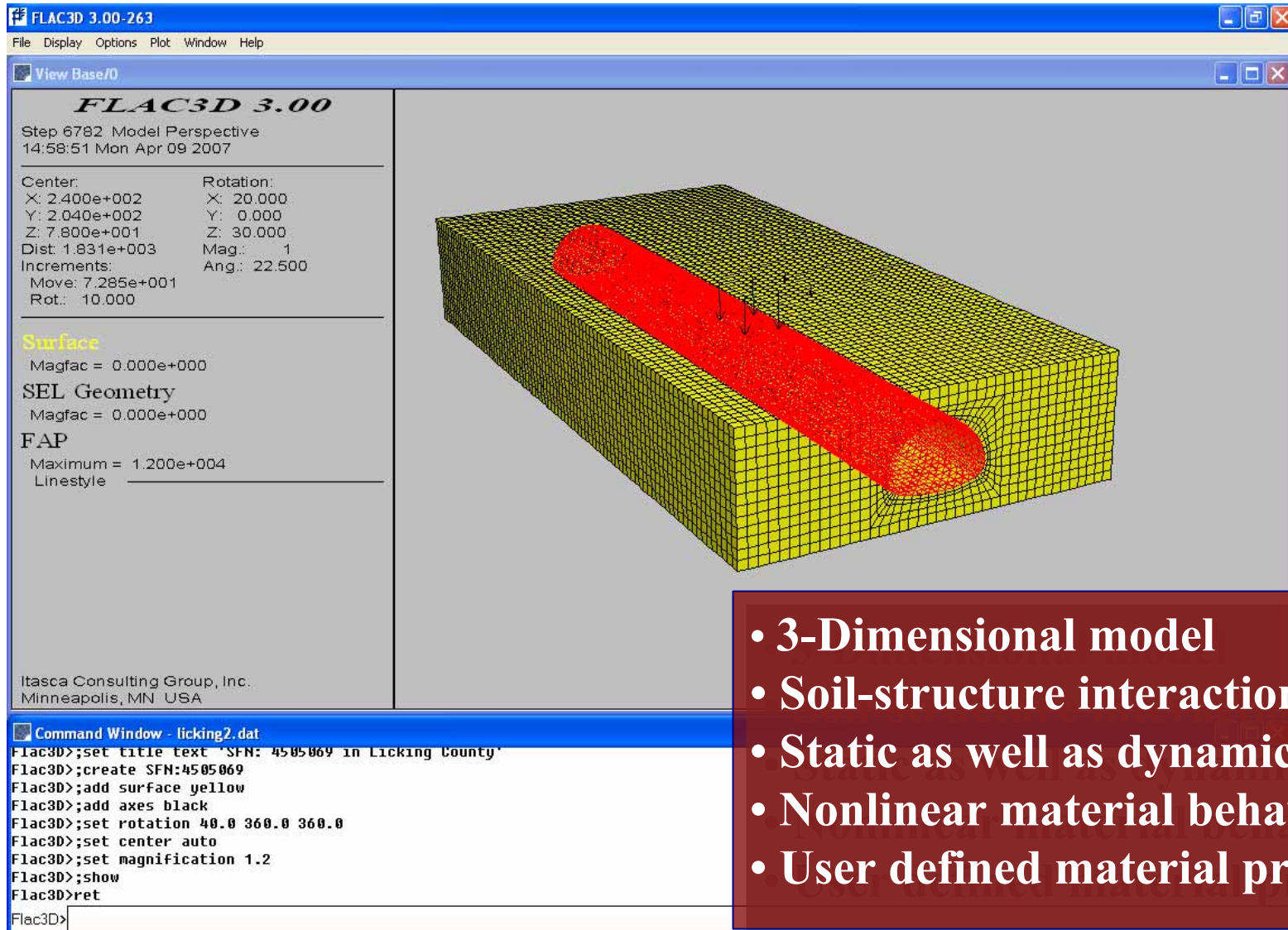
Maximum displacement measured in the field: **0.81 mm**

Analytical vs. Experimental Results



- Calculated deflections are much larger than measured deflections.
- Trend is similar.

FLAC-3D MODELING PROGRESS



FLAC3D 3.00
Step 6782 Model Perspective
14:58:51 Mon Apr 09 2007

Center:	Rotation:
X: 2.400e+002	X: 20.000
Y: 2.040e+002	Y: 0.000
Z: 7.800e+001	Z: 30.000
Dist: 1.831e+003	Mag.: 1
Increments:	Ang.: 22.500
Move: 7.285e+001	
Rot.: 10.000	

Surface
Magfac = 0.000e+000

SEL Geometry
Magfac = 0.000e+000

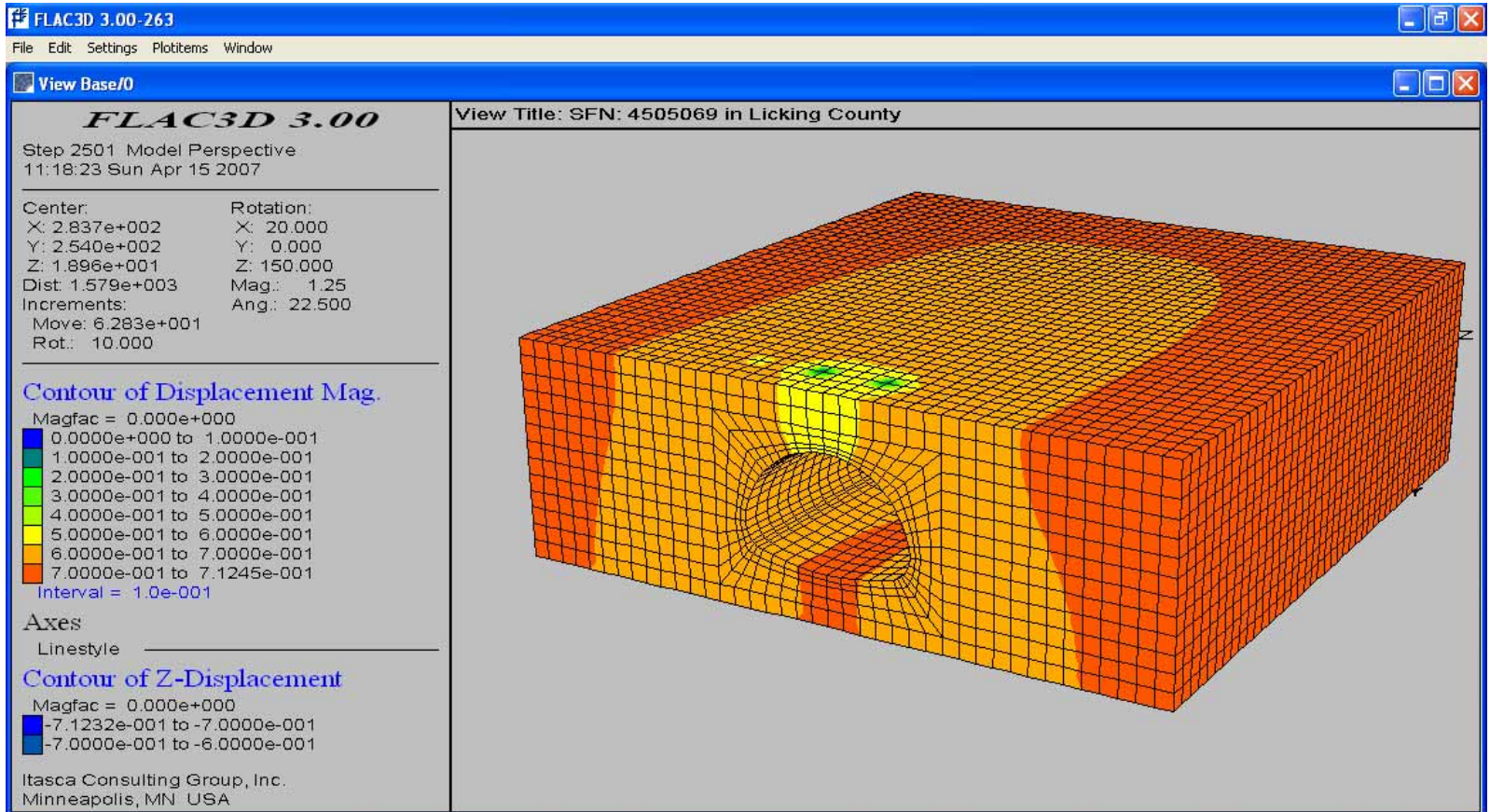
FAP
Maximum = 1.200e+004
Linestyle

Itasca Consulting Group, Inc.
Minneapolis, MN USA

```
Command Window - licking2.dat
Flac3D>;set title text 'SFN: 4505069 in Licking County'
Flac3D>;create SFN:4505069
Flac3D>;add surface yellow
Flac3D>;add axes black
Flac3D>;set rotation 40.0 360.0 360.0
Flac3D>;set center auto
Flac3D>;set magnification 1.2
Flac3D>;show
Flac3D>ret
Flac3D>
```

- 3-Dimensional model
- Soil-structure interaction
- Static as well as dynamic analysis
- Nonlinear material behavior
- User defined material properties

PRELIMINARY RESULTS



- Elastic analysis of LIC-79-2276
- Maximum calculated deflection = 3.16 mm
- Maximum measured deflection = 0.23 mm

Evaluation of ODOT's CMP-Excel Program

- **Used for evaluation of corrugated steel culverts**
- **Provides a “Load rating (LR)” factor (smaller of):**
 - *Rating Factor based on wall strength (RFW)*
 - *Rating Factor based on minimum cover (RFC)*
- ➔ **For shallow culverts, RFW does not affect LR because RFC is very small. Thus,**
 - *Material type does not matter*
 - *Magnitude of wall thrust does not matter*
 - *External loads do not matter*
- ➔ **A better method is needed to evaluate the relation between RFW and RFC**

Current Work: Evaluation of CMP-Excel

Using test data, evaluate CMP-Excel:

- Compare the culvert response with load rating factors:
 - Use measured deflections, and moments and thrust forces calculated from measured strains
- Evaluate the effectiveness and significance of each parameter in CMP-Excel:
 - Rating Factor based on wall thrust
 - Rating Factor based on minimum cover

SUMMARY

Experimental Research:

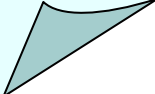
- 38 culverts have been tested
- Test data and critical parameters have been evaluated

Analytical Research:

- CANDE: Modeling and analysis completed
 - FLAC: Modeling and preliminary analysis completed
 - CMP-Excel: A better method is needed
- 

CONCLUSIONS

Experimental Research:

- Culvert deflections decrease nonlinearly with increasing soil backfill height
 - Maximum deflections are close to zero for deep culverts (backfill height > 13 ft or 4 m)
 - Substantial increase in strains and deflections in shallow culverts (backfill height ≤ 8 ft or 2.5 m)
 - It is difficult to predict the deflections in shallow culverts (age, soil type, condition may be as important as backfill and loading)
 - Maximum dynamic deflections are 10-30% less than maximum static deflections
- 

CONCLUSIONS

Analytical Research:

- **CANDE:** Predicted deflections are too large
- **FLAC:** More analyses with refined models are needed
- **CMP-Excel:** Better/simplified methods are needed

THANK YOU

- **QUESTIONS?**
- **COMMENTS?**

Load rating factor in CMP-Excel

Operating Load Rating Factor

a. Rating factor based on wall strength

$$RFW = \frac{T_{cap} - 1.95T_E}{1.3T_{L+i}}$$

where,

T_{cap} = thrust capacity of the wall

T_E = pipe wall thrust due to earth cover

T_{L+i} = pipe wall thrust due to live load plus impact

b. Rating factor based on minimum cover requirement

$$RFC = \frac{H^2}{C \cdot h^2}$$

where,

H = backfill height

h = the AASHTO minimum cover

C = $2.36H / s + 0.528 \leq 1.0$

s = span length