

**DESIGN NOTES**

1. DESIGN SPECIFICATIONS: "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1996, INCLUDING THE 1997 THROUGH 2001 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

**2. DESIGN DATA:**

- LIVE LOADING - HS25 AND THE ALTERNATE MILITARY LOADING
- SUPERIMPOSED DEAD LOADS - ASPHALT OVERLAY - 3 1/2" THICK (AVG.)  
RAILING WEIGHT - 90 PLF PER RAIL (TST-1-99)  
FWS - 60 PSF
- DEAD LOAD - DIAPHRAGM WEIGHT IS BASED ON 3'-0" LONG DIAPHRAGMS.
- CONCRETE - MIN. COMPRESSIVE STRENGTH AT 28 DAYS  $f'_c = 7000$  P.S.I.  
MIN. COMPRESSIVE STRENGTH AT TIME OF INITIAL PRESTRESS  $f'_c = 5000$  P.S.I.
- REINFORCING STEEL - GRADE 60  
MINIMUM YIELD STRENGTH 60,000 P.S.I.
- PRESTRESSING STEEL - ASTM A416  
1/2" DIAMETER  
 $A_s = 0.167$  SQ. IN.  
 $f_s = 270,000$  P.S.I.  
 $E_s = 28,500$  K.S.I.  
 $R_H = 70\%$   
INITIAL STRESS  $0.75 f_s = 202,500$  P.S.I.  
INITIAL TENSION LOAD = 33,818 LBS/STRAND

3. PRESTRESS LOSSES HAVE BEEN COMPUTED IN ACCORDANCE WITH AASHTO ARTICLE 9.16.2. TOTAL LOSSES DETERMINED BY THIS METHOD MAY BE EXPRESSED AS  $\Delta f_s = 11,175 + (25,650/E_c + 11.4) f_{cir} - 6.65 f_{cbs}$

**4. INTERMEDIATE DIAPHRAGMS:**

- SPAN  $\leq 50$  FT. USE ONE DIAPHRAGM
- 50 FT.  $<$  SPAN  $\leq 75$  FT. USE TWO DIAPHRAGMS
- SPAN  $> 75$  FT. USE THREE DIAPHRAGMS

5. CAMBER DATA GIVEN IS THE CALCULATED CAMBER AT TIME OF PAVING (1.8B - 1.85C), WHERE B - CAMBER DUE TO PRESTRESSING AT RELEASE AND C - DEFLECTION DUE TO WEIGHT OF BEAM INCLUDING DIAPHRAGMS. D - CALCULATED DEFLECTION AT MIDSPAN DUE TO A 3 1/2" THICK ASPHALT WEARING SURFACE AND TWO (2) BRIDGE RAILS WEIGHING 90LB/FT. PER RAIL. THE VALUE SHOWN IS THE MAXIMUM INITIAL SUPERIMPOSED DEAD LOAD DEFLECTION FOR GROUP F ROADWAY WIDTHS.

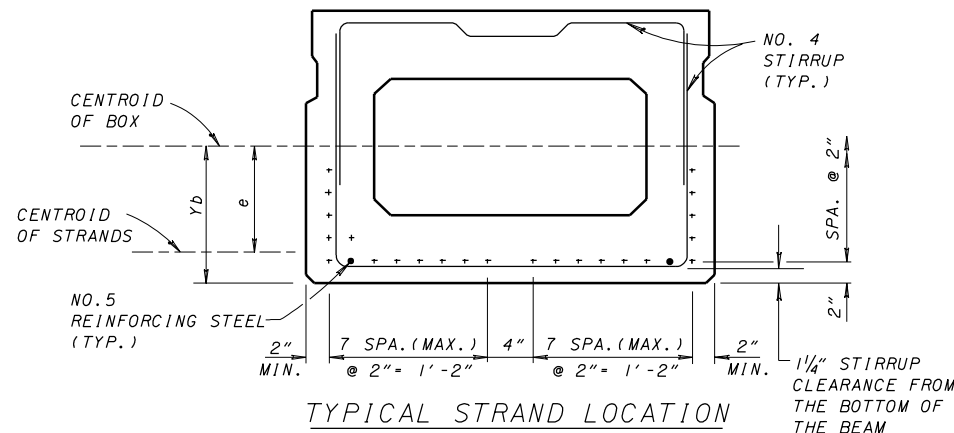
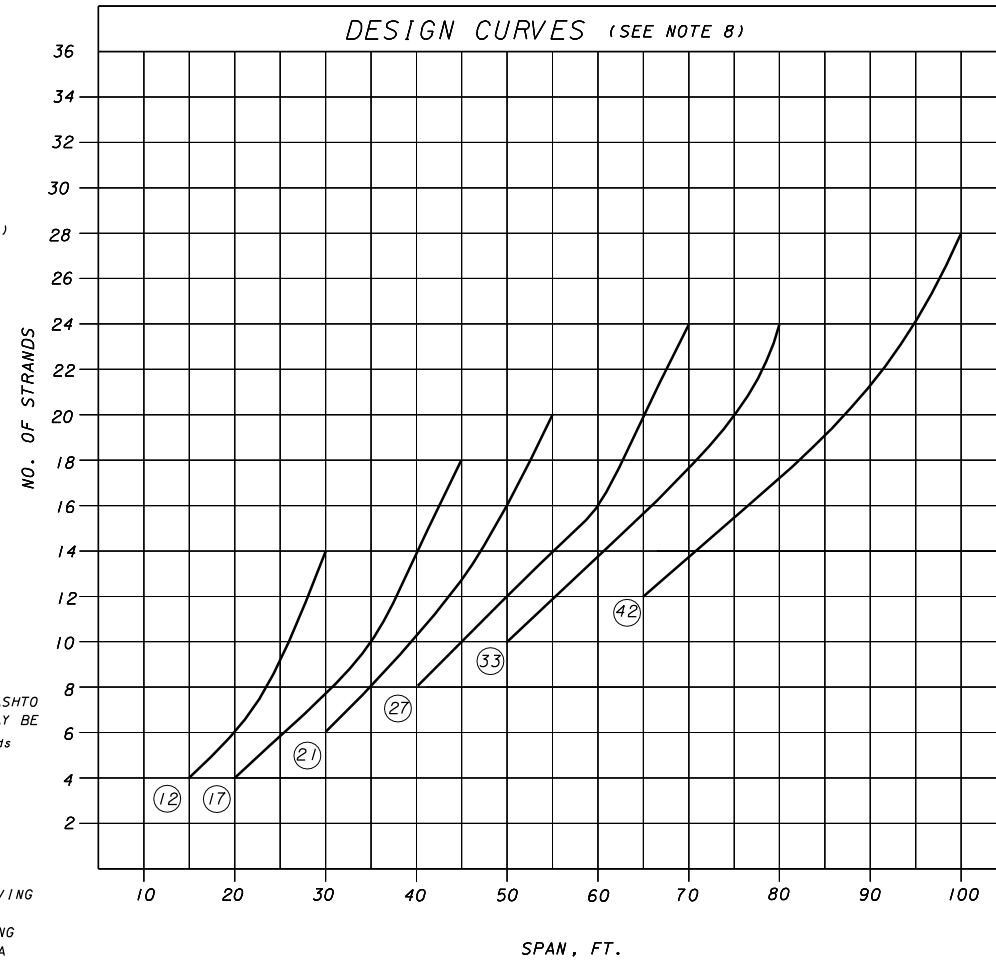
6. THIS DRAWING PROVIDES INFORMATION FOR THE DESIGNER AND IS NOT INTENDED FOR USE AS A STANDARD DRAWING. REFERENCE SHALL BE MADE TO STANDARD DRAWING PSBD-1-93 FOR DETAILS OF BEAMS.

7. ROADWAY WIDTH: THE BEAMS ON THIS SHEET ARE DESIGNED FOR THE FOLLOWING ROADWAY WIDTHS, MEASURED BETWEEN FACE OF BRIDGE RAILS:

GROUP F  
63 FT.  $<$  WIDTH  $\leq 72$  FT.

8. INTERPOLATION: THE DESIGN CURVES SHALL BE USED TO DETERMINE THE NUMBER OF STRANDS REQUIRED FOR BEAM SPANS NOT SPECIFICALLY LISTED IN THE DESIGN DATA TABLE. HOWEVER, THE NUMBER OF STRANDS USED SHALL ALWAYS BE ROUNDED UP TO THE NEAREST EVEN NUMBER. THE REMAINING DESIGN DATA MAY BE OBTAINED FROM THE TABLE BY USING STRAIGHT LINE INTERPOLATION BETWEEN GIVEN VALUES. DO NOT EXTRAPOLATE BEYOND THE CURVES OR GIVEN DATA.

9. NARROW ROADWAY: THESE DESIGNS SHALL NOT BE USED FOR ROADWAY WIDTHS LESS THAN 24 FT. SPECIAL DESIGN IS REQUIRED FOR ROADWAY WIDTHS LESS THAN 24 FT.



STRANDS SHALL BE PLACED PER BDM 302.5.1.2.b AND SHALL BE DISTRIBUTED SYMMETRICALLY OVER THE BEAM WIDTH. STRAND PATTERN AND THE DEBONDED LENGTHS SHALL BE SYMMETRICAL ABOUT VERTICAL  $\bar{C}$  OF BEAM. DEBONDED STRANDS SHALL BE IN BOTTOM LAYER. LENGTH OF STRANDS TO BE DEBONDED IS MEASURED FROM ENDS OF BEAM. TWO BOTTOM REINFORCING BARS SHALL BE LOCATED AT THE CORNER OF THE STIRRUPS. A LAP OF 3'-3" FOR #5 BARS AND 4'-0" FOR #6 BARS SHOULD BE PROVIDED WITHIN THE OUTER QUARTER OF THE SPAN, IF NEEDED.

**GROUP F DESIGN DATA (SEE NOTE 7)**

BOX	SPAN c/c BRG. FT.	MID-SPAN e in.	NO. OF STRANDS	STRAND LOCATION FROM BOTTOM OF BOX								NUMBER AND LENGTH OF STRANDS DEBONDED		TENSILE BARS AT BOTTOM		TENSILE BARS AT TOP				CAMBER/DEFLECTION DATA IN. (SEE NOTE 5)							
				2"	4"	6"	8"	10"	12"	14"	1'-6"	2'-6"	3'-6"	4'-6"	NO.	SIZE	FULL LENGTH		ADDITIONAL BARS EACH END		B	C	1.8B-1.85C	D			
				NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.			
B12-36	15	3.96	4	4													2	5	4	5	-	-	-	0.094	0.023	0.127	0.006
	20	3.96	6	6													2	5	4	5	-	-	-	0.250	0.072	0.317	0.019
	25	3.56	10	8	2												2	5	4	5	-	-	-	0.581	0.176	0.720	0.047
	30	3.39	14	10	4												2	5	4	5	-	-	-	1.098	0.366	1.299	0.098
B17-36	20	6.42	4	4													4	5	4	5	-	-	-	0.101	0.029	0.128	0.007
	25	6.42	6	6													2	5	4	5	-	-	-	0.236	0.070	0.295	0.017
	30	6.42	8	8													2	5	4	5	-	-	-	0.450	0.144	0.544	0.036
	35	6.42	10	10													2	5	4	5	-	-	-	0.761	0.264	0.881	0.067
	40	6.13	14	12	2												2	5	4	5	-	-	-	1.312	0.449	1.531	0.114
B21-36	45	5.98	18	14	4												2	5	4	5	-	-	-	2.041	0.716	2.349	0.183
	30	8.39	6	6													2	5	4	5	-	-	-	0.248	0.091	0.278	0.020
	35	8.39	8	8													2	5	4	5	-	-	-	0.448	0.168	0.496	0.037
	40	8.39	12	12													2	5	4	5	2	5	4'-3"	0.864	0.285	1.028	0.064
	45	8.10	14	12	2												2	5	4	5	2	5	4'-0"	1.227	0.454	1.369	0.102
B27-36	50	7.89	16	12	4												2	5	4	5	2	5	4'-0"	1.680	0.688	1.751	0.155
	55	7.59	20	14	4	2											2	5	4	5	-	-	-	2.412	1.032	2.432	0.227
	40	11.36	8	8													2	5	4	5	-	-	-	0.408	0.167	0.425	0.033
	45	11.36	10	10													2	5	4	5	2	5	3'-9"	0.641	0.267	0.660	0.052
	50	11.36	12	12													2	5	4	5	2	5	5'-0"	0.944	0.403	0.954	0.079
	55	11.36	14	14													2	5	4	5	4	5	6'-0"	1.325	0.610	1.257	0.116
B33-36	60	11.11	16	14	2												2	5	4	5	4	5	6'-0"	1.755	0.857	1.574	0.165
	65	10.56	20	14	4	2											2	5	4	5	2	5	5'-6"	2.420	1.173	2.186	0.227
	70	9.53	24	14	4	2	2	2									2	5	4	5	-	-	-	3.024	1.569	2.541	0.305
	50	14.28	10	10													2	5	4	5	2	5	4'-6"	0.592	0.267	0.572	0.047
	55	14.28	12	12													2	5	4	5	2	5	5'-9"	0.855	0.406	0.788	0.069
	60	14.28	14	14													2	5	4	5	4	5	7'-0"	1.181	0.571	1.069	0.098
	65	14.03	16	14	2												2	5	4	5	4	5	7'-3"	1.549	0.780	1.345	0.135
	70	13.84	18	14	4												2	5	4	5	4	5	7'-3"	1.985	1.043	1.643	0.181
B42-36	75	13.48	20	14	4	2											2	5	4	5	4	5	7'-0"	2.460	1.366	1.901	0.239
	80	12.45	24	14	4	2	2	2									2	5	4	5	2	5	5'-3"	3.082	1.853	2.120	0.309
	65	18.72	12	12													2	5	4	5	4	5	6'-9"	0.849	0.493	0.616	0.072
	70	18.72	14	14													2	5	4	5	4	5	8'-0"	1.143	0.658	0.840	0.097
	75	18.47	16	14	2												2	5	4	5	6	5	8'-6"	1.474	0.861	1.060	0.128
	80	17.83	18	12	4	2											2	5	4	5	6	5	8'-3"	1.817	1.149	1.145	0.166
	85	17.92	20	14	4	2											2	5	4	5	6	5	9'-0"	2.280	1.454	1.414	0.212
90	17.45	22	14	4	2	2										2	5	4	5	6	5	8'-6"	2.731	1.815	1.558	0.266	
95	16.89	24	14	4	2	2	2									2	5	4	5	4	5	7'-3"	3.209	2.240	1.632	0.330	
100	15.58	28	14	4	2	2	2	2	2	2	2					2	5	4	5	4	5	6'-6"	3.810	2.735	1.798	0.406	

▲ - LENGTH MEASURED FROM ENDS OF BEAM

BOX	A IN. <sup>2</sup>	I IN. <sup>4</sup>	Yb IN.	S <sub>t</sub> IN. <sup>3</sup>	S <sub>b</sub> IN. <sup>3</sup>
B12 - 36	423.8	5122	5.96	848	859
B17 - 36	426.3	13840	8.42	1613	1644
B21 - 36	479.8	24893	10.39	2346	2396
B27 - 36	539.8	48647	13.36	3567	3641
B33 - 36	594.5	82048	16.28	4907	5040
B42 - 36	684.5	152479	20.72	7165	7359

REVISIONS	STATE OF OHIO DEPARTMENT OF TRANSPORTATION OFFICE OF STRUCTURAL ENGINEERING	6/6
ORIGINAL DESIGN PREPARED BY: URS CORPORATION - OHIO		
<b>DESIGN DATA FOR PRESTRESSED CONCRETE BRIDGE GROUP F ROADWAY WIDTH NON-COMPOSITE 36" ADJACENT BOX BEAMS WITH STRAIGHT STRANDS</b>		
DESIGNED MDP	DRAWN SFW	CHECKED MAC
REVIEWED RSC	DATE 12/20/02	